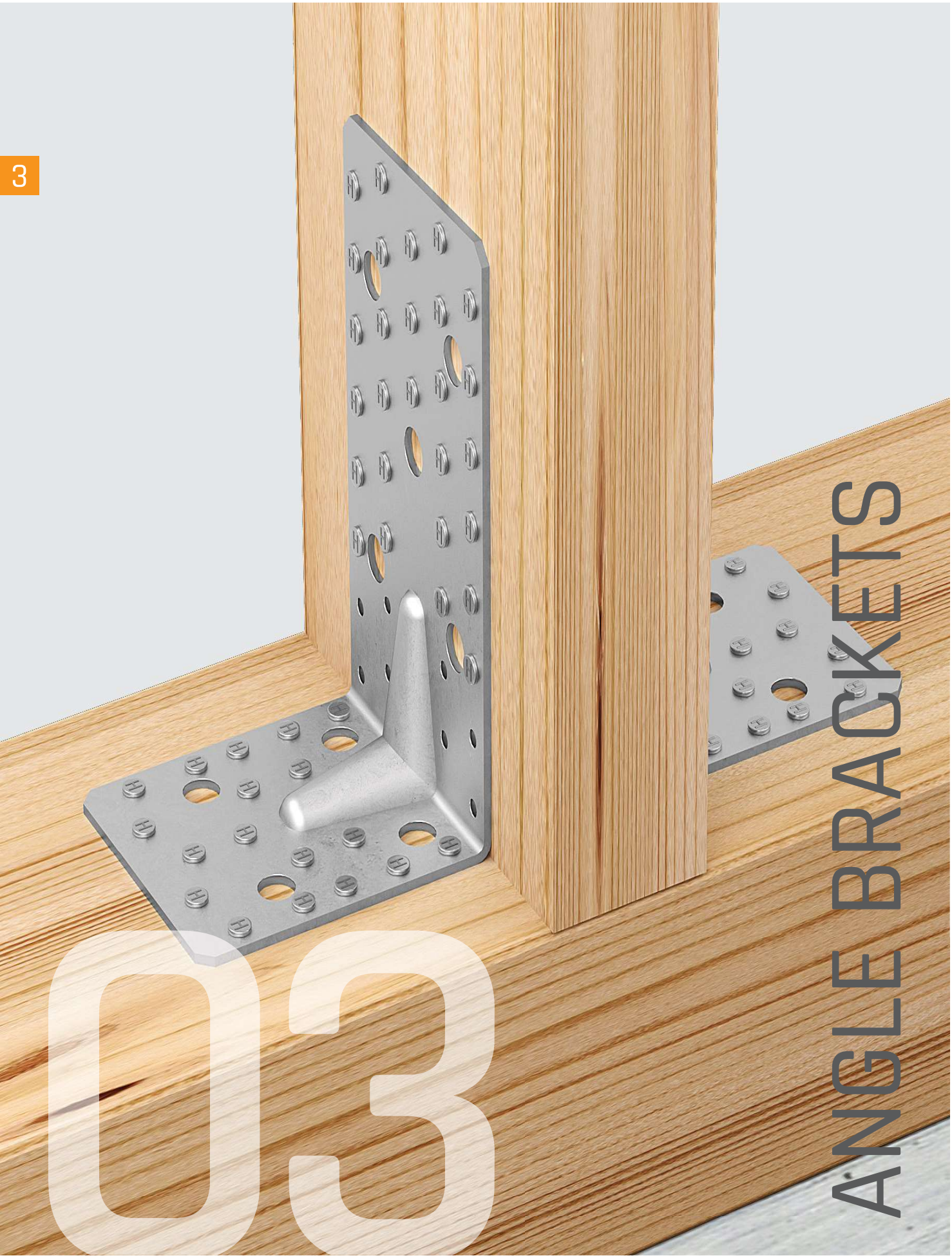


03 ANGLE BRACKETS

ANGLE BRACKETS

ASSORTMENT

	Height [mm]	Length [mm]	Width [mm]	Basics Statik & Diagramme from page	Products & Statik from page	Products Made of V4A from page
ANGLE BRACKET TOP 80/120	80-120	60	55	110	120	
ANGLE BRACKET 70X70X2.0	70	70	55	110	122	293
ANGLE BRACKET 70X70 GREENLINE	70	70	55	110	122	
ANGLE BRACKET TOP KR90E	95	85	65	110	124	
ANGLE BRACKET 90X90X2.5	90	90	65	110	128	293
ANGLE BRACKET 90X90 GREENLINE	90	90	65	110	126	
ANGLE BRACKET 100X100X3.0	100	100	90	110	130	293
ANGLE BRACKET 100X100 GREENLINE	105	105	90	110	130	
STRUT CONNECTOR 135 DEGREES	90-100	90-100	65-90		132	
ANGLE BRACKET TYPE 110/170L	170	110	95	110	132	
ANGLE BRACKET KR 3 MM	95-285	88	65	110	134	
ANGLE BRACKET KR 4 MM	95-285	88	65	110	134	
ANGLE BRACKET TYPE 50/80	90	50	50-80	110	138	
ANGLE BRACKET TYPE 110	90	50	110	110	138	
ANGLE BRACKET TYPE 55/80	80	60	55	110	140	
ANGLE BRACKET TYPE 60/100	100	60	60	110	140	
CONSOLE ANGLE	120-200	54	60	110	142	
ANGLE BRACKET TYPE 40/45	50-90	50-90	40-45	110	144	
ANGLE BRACKET TYPE 40/90	90	90	40	110	146	294
ANGLE BRACKET TYPE 40/120	120	95	40	110	146	
ANGLE BRACKET TYPE 692	65	65	90	110	146	

ANGLE BRACKETS

ASSORTMENT

	Height	Length	Width	Basics Statics & Diagrams from page	Products & Statics from page	Products Made of V4A from page
NAIL PLATE BRACKET	40-200	40-100	20-100		148	294
EXTRA THICK 4 MM	130-160	70	80-100	110	150	
MOUNTING BRACKET	90	60	60	110	152	
ENTRANCE DOOR BRACKET	70	30	60		158	
Z-CONNECTOR	40	75	30		158	
CHAIR BRACKET	25-120	25-120	15-20		159	
ANGLE BRACKET THICK 3-5 MM	40-180	40-180	20		159	
CONCRETE BRACKET	75-150	75	60		156	
CORNER ANGLE BRACKET	40	40	100-250		158	

CE symbol

Steel with indication of the steel quality and galvanisation

Stainless steel with material number

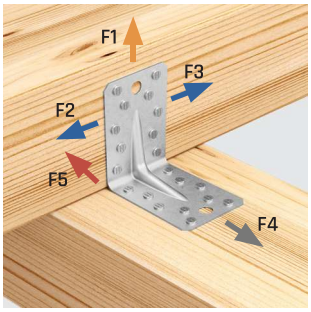
Timber/timber connection

Timber/concrete-connection

Usage class 1
Moisture content in the building materials that corresponds to a temperature of 20° C and a relative humidity of the ambient air that only exceeds a value of 65% for a few weeks per year, e.g. in the case of buildings that are closed on all sides and heated.
Comment: In UC 1, the average moisture content of most softwoods does not exceed 12 %.

Usage class 2
Moisture content in the building materials that corresponds to a temperature of 20° C and a relative humidity of the ambient air that only exceeds a value of 85% for a few weeks per year, e.g. in the case of open buildings covered by a roof.
Comment: In UC 2, the average moisture content of most softwoods does not exceed 20 %.

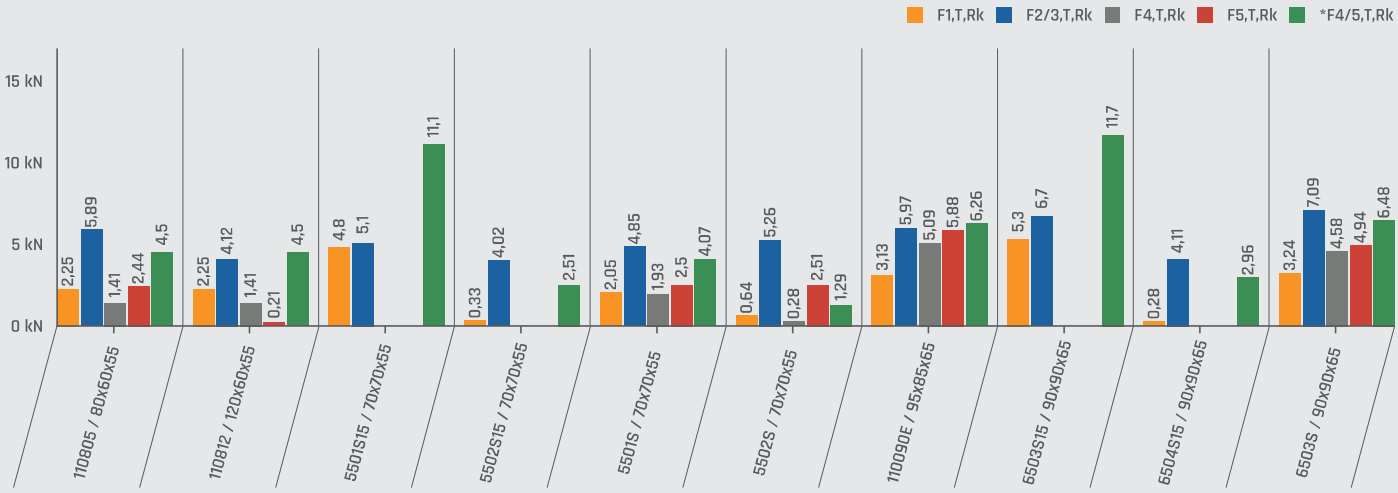
Usage class 3
Includes climatic conditions that lead to higher moisture contents than in UC 2, e.g. structures that are exposed to the weather without protection.
Eurocode 5 / DIN EN 1995-1-1 section 2.3.1.3

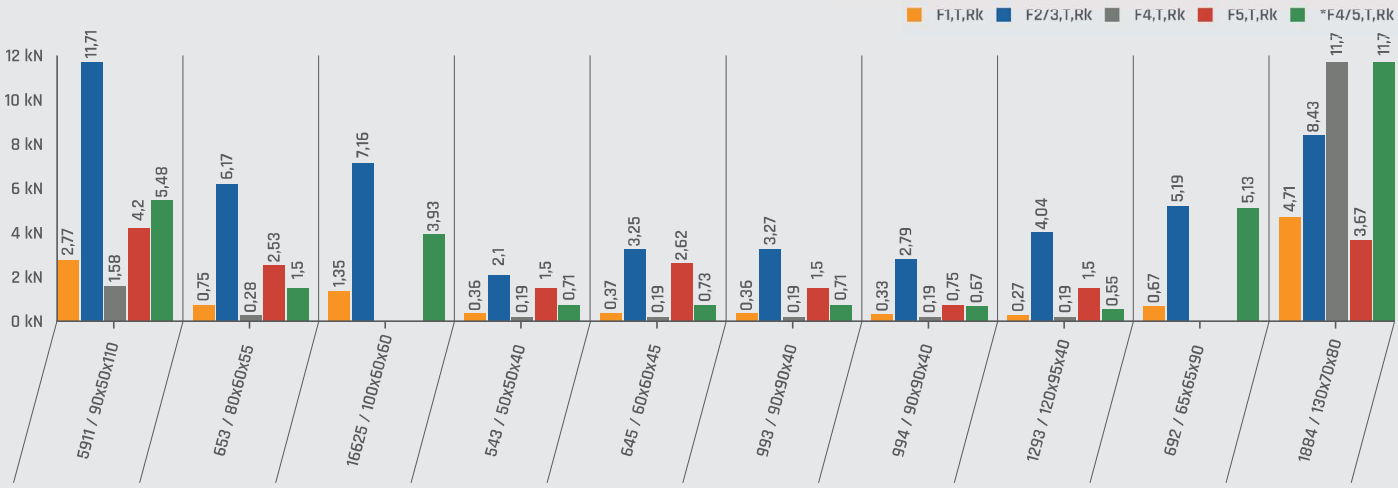


ANGLE BRACKETS

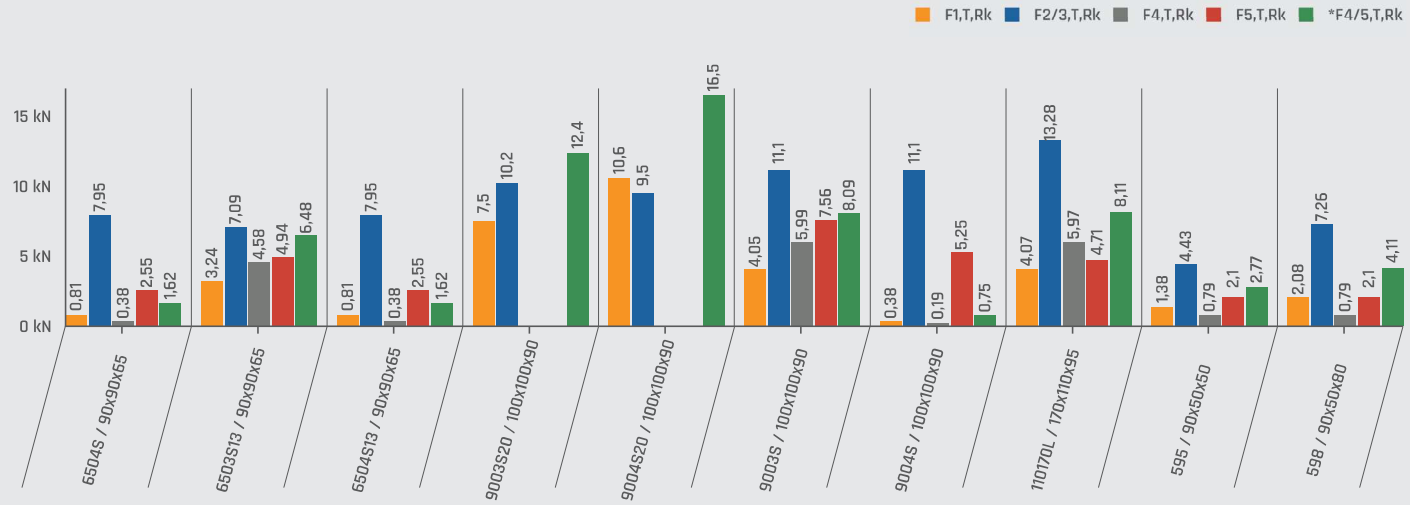
STATICS DIAGRAM

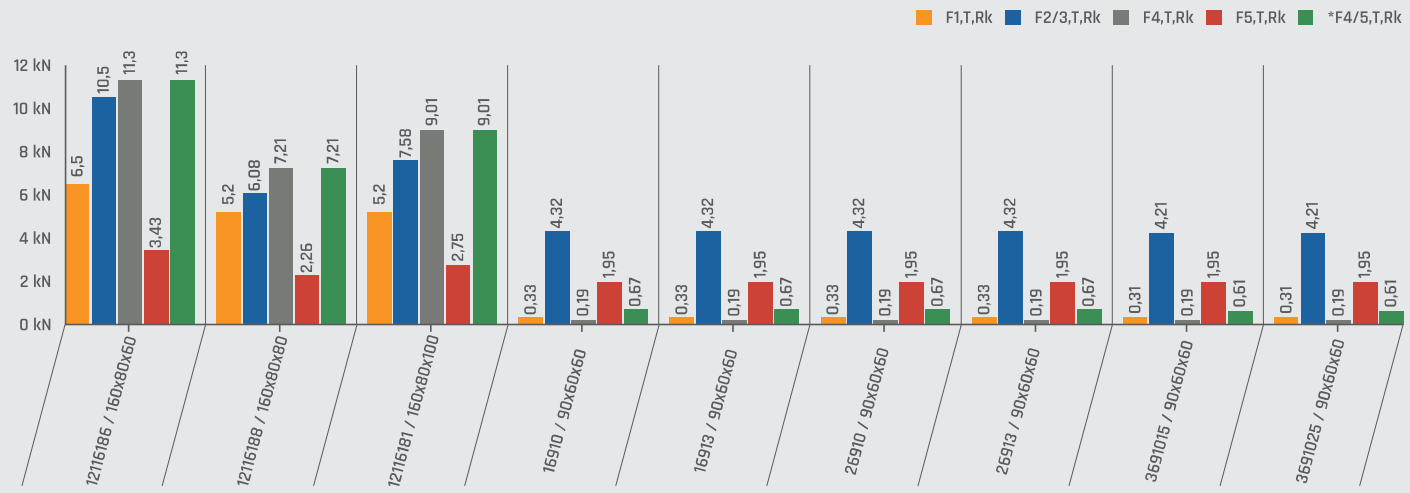
3





3


F1,T,Rk
F2/3,T,Rk
F4,T,Rk
F5,T,Rk
*F4/5,T,Rk
15 kN
10 kN
5 kN
0 kN
0,81
7,95
0,38
2,55
1,62
3,24
7,09
4,58
4,94
6,48
0,81
7,95
0,38
2,55
1,62
7,5
10,2
12,4
10,6
9,5
16,5
4,05
11,1
5,99
7,55
8,09
0,38
11,1
0,19
5,25
0,75
4,07
13,28
5,97
4,71
8,11
1,38
4,43
0,79
2,1
2,77
2,08
7,26
0,79
2,1
4,11
6504S / 90x90x65
6503S13 / 90x90x65
6504S13 / 90x90x65
9003S20 / 100x100x90
9004S20 / 100x100x90
9003S / 100x100x90
9004S / 100x100x90
110170L / 170x110x95
595 / 90x50x50
598 / 90x50x80


F1,T,Rk
F2/3,T,Rk
F4,T,Rk
F5,T,Rk
*F4/5,T,Rk
12 kN
10 kN
8 kN
6 kN
4 kN
2 kN
0 kN
6,5
10,5
11,3
3,43
11,3
5,2
6,08
7,21
2,25
7,21
5,2
7,58
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2,75
9,01
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1,95
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4,32
0,19
1,95
0,67
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1,95
0,67
0,31
4,21
0,19
1,95
0,61
0,31
4,21
0,19
1,95
0,61
12116186 / 160x80x60
12116188 / 160x80x80
12116181 / 160x80x100
16910 / 90x60x60
16913 / 90x60x60
26910 / 90x60x60
26913 / 90x60x60
3691015 / 90x60x60
3691025 / 90x60x60

ANGLE BRACKETS

TECHNICAL FEATURES

3

Geometry

H	Height (mm)
L	Length (mm)
B	Width (mm)
S	Material thickness (mm)

Tables

n_a	Number of connecting elements
NB	Nail pattern
Full	Maximum number of connecting elements
Partial	Minimum number of connecting elements

Timber connecting element

$Ø_{[mm]}$	Diameter of connecting element
$L_{(mm)}$	Length of connecting element
	Grain direction in the wood component

Connecting element concrete/ steel

Bo	Dowels/bolts

Load directions

F_1	Force at a right angle to the connector level, lifting force
F_2	Force in rod direction
F_3	Force in rod direction
F_4	Force in direction of the angle bracket
F_5	Force away from the angle bracket

CE symbol

Steel with indication of the steel quality and galvanisation

Stainless steel with material number

Timber/timber connection

Timber/concrete-connection

Usage class 1
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Comment: In UC 2, the average moisture content of most softwoods does not exceed 20 %.

Usage class 3
Includes climatic conditions that lead to higher moisture contents than in UC 2, e.g. structures that are exposed to the weather without protection.
Eurocode 5 / DIN EN 1995-1-1 section 2.3.1.3

Design

$F_{1,Ed}$	Design load for load direction 1 in kN
$F_{2/3,Ed}$	Design load for load direction 2 or 3 in kN
$F_{4,Ed}$	Design load for load direction 4 in kN
$F_{5,Ed}$	Design load for load direction 5 in kN
$F_{4/5,Ed}$	Design load for load direction 4 or 5 in kN
$F_{1,Rk}$	Characteristic value of the load capacity in load direction 1 for one or for two angle brackets in kN
$F_{2/3,Rk}$	Characteristic value of the load capacity in load direction 2 or 3 for one or for two angle brackets in kN
$F_{4,Rk}$	Characteristic value of the load capacity in load direction 4 for one angle bracket in kN
$F_{5,Rk}$	Characteristic value of the load capacity in load direction 5 for one angle bracket in kN
$F_{4/5,Rk}$	Characteristic value of the load capacity in load direction 4 or 5 for two angle brackets in kN
$F_{1,Rd}$	Design value of the load capacity in load direction 1 for one or for two angle brackets in kN
$F_{2/3,Rd}$	Design value of the load capacity in load direction 2 or 3 for one or for two angle brackets in kN
$F_{4,Rd}$	Characteristic value of the load capacity in load direction 4 for one angle bracket in kN
$F_{5,Rd}$	Characteristic value of the load capacity in load direction 5 for one angle bracket in kN
$F_{4/5,Rd}$	Characteristic value of the load capacity in load direction 4 or 5 for two angle brackets in kN
$F_{i,Ed}$	Design value of load capacity for one or two angle brackets for the respective load direction "i" (i = 1 to 5) in kN
$F_{i,Rk,T}$	Characteristic value of the load-bearing capacity of the sheet steel-timber connection with GH threaded/anchor nails for the 'i' (i = 1 to 5) in kN respective load direction 'i' (i = 1 to 5) in kN
$F_{i,Rk,S}$	Characteristic value for steel load capacity of the bracket (table value **„S"** oder $F_{Rk,S}$ bzw. $F_{Rd,S}$) for the respective load direction "i" (i = 1 to 5) in kN
k_{mod}	Modification factor for load impact duration and usage class
$\gamma_{M,T}$	Partial safety factor for timber (for Germany: 1.3)
$\gamma_{M,S}$	Partial safety factor for steel for cross-section stresses (for Germany: 1.0)

Dowel design

$k_{i,t,ax}$	Coefficient for calculating the axial load-bearing capacity per bolt, for connecting the bracket to concrete or steel components for the respective Load direction "i" (i = 1 to 5)
$k_{i,t,v}$	Coefficient for calculating the shear load-bearing capacity per bolt, for connecting the bracket to concrete or steel components for the respective Load direction "i" (i = 1 to 5)
$F_{i,Ed}$	Design load on one or two angle brackets for the respective load direction "i" (i = 1 to 5) in kN
$F_{i,Rd}$	Design value of load capacity on one or two angle brackets for the respective load direction "i" (i = 1 to 5) in kN
$F_{i,Ed,B}$	Design value for one bolt or one anchor for the respective load direction "i" (i = 1 to 5) in kN
$F_{i,Rd,B}$	Design value of the load-bearing capacity of the entire connection to concrete or steel with bolts or anchors for the respective Load direction 'i' (i = 1 to 5) in kN (calculation must be carried out separately and is based on the respective approval and standard of the bolts or anchors)

ANGLE BRACKETS

APPLICATIONS

3

Application:
Timber/timber; timber/concrete, steel connections

Materials:

350 GD Z275 | 250 GD Z275 | 235 JR feuerverzinkt | A4 1.4571

Material thicknesses:
1.5 / 2.0 / 2.5 / 3.0 / 4.0 / 6.0 / 8.0 mm
More on request.

Sleeper **Style**

Sleeper **Style**

Sleeper **Concrete**

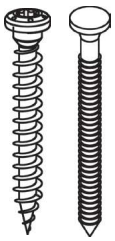
Cross

Sleeper **Concrete**

BSH **Concrete**

For use in usage classes

NKL 1 | NKL 2 | NKL 3



Connecting element:

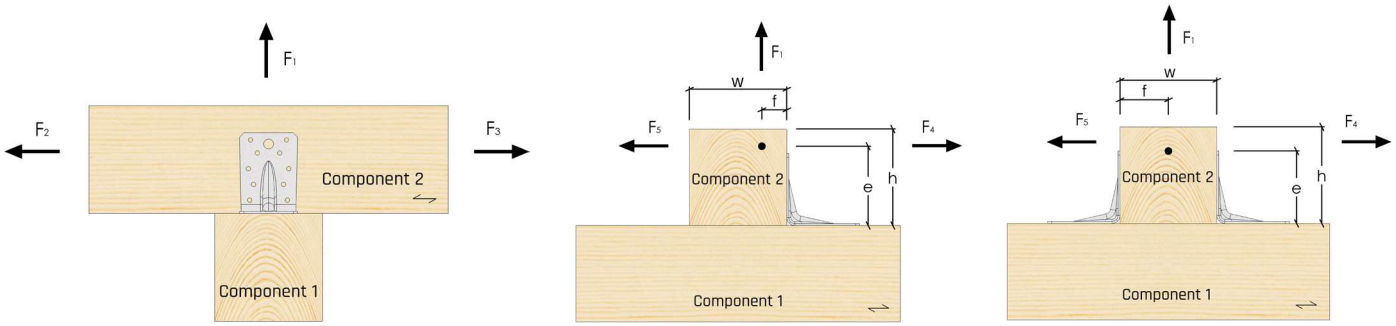
GH threaded nails 4.0 x 35 / 40 / 50 / 60 / 75 / 100 mm
GH screws 5.0 x 25 / 35 / 40 / 50 / 60 / 70 mm

Bolt, dowel or concrete anchor M10, M12

Connecting elements from page 274

ANGLE BRACKETS

LOAD DIRECTIONS



Load F_1:

For the load-bearing capacity of a bracket, the load is applied at spacing f from the contact surface between the bracket and the timber beam is applied (Figure 2). If it is assumed that the timber component is prevented from rotating or if two console angles are arranged, then the eccentricity is f = 0.

Load $F_{2/3}$

Calculation of the load-bearing capacity of one or two angle brackets that are loaded with a force in the direction of the axis of component 2 (Fig. 1).

Load F_4 / F_5 / $F_{4/5}$

In all three cases, the load is applied at a distance e from the contact surface between component 1 and component 2 (Figure 2).
The load cases are considered as a combination of two base load cases.
The first base load case is the lateral load with forces n F_4, F_5 or $F_{4/5}$ with e = 0.
For the arrangement with an angle bracket, the rotation of component 2 is taken into account.
For the arrangement with two angle brackets, the rotation of component 2 is prevented and the load on the beam due to the moment is calculated as the lifting force $F_1 = F_{4/5}$ x e/w. w is thereby the width of component 2

Nail patterns

Partial and full nail fitting or partial and full screw fitting
See nail pattern for the product

Connection over intermediate layers

The characteristic load capacities for the connection with angle brackets indicated in the tables also apply to an intermediate layer between the angle bracket and timber component. The following conditions must be met:

- The intermediate layer must be connected to the timber component without being able to be moved.
- The insert depth of the profiled area of the nail or the thread length of the screw in the timber component must be the same or greater.
- For this, a correspondingly long connecting element must be used.
- The characteristic perforation strength $f_{h,k}$ of the connecting element in the intermediate layer must be the same of greater.

Determination of load capacity

The load capacity of connections with angle brackets $F_{i,Rd}$ for the respective load direction "i" (i = 1 to 5) corresponds to the smallest value of:

- The load capacity of the steel sheet-timber connection with GH threaded/anchor nails $F_{i,Rd,T}$
- Steel load capacity of the bracket $Fi_{,Rd,S}$,
- Load-bearing capacity of the connection to concrete or steel with bolts or anchors under consideration of the coefficient $k_{i,t,ax}$ or $k_{i,t,v}$, see section 4.

$$F_{i,Rd} = \min\left\{\frac{k_{mod} \cdot F_{i,Rk,T}}{\gamma_{M,T}}; \frac{F_{i,Rk,S}}{\gamma_{M,S}}; F_{i,Rd,B}\right\}$$

The increased load capacity values of the GH threaded/anchor nails according to ETA-13/0523 apply.
The values of EN14592 apply to angle brackets made of stainless steel.

Connection timber- concrete/ steel

The design load of the respective load direction "i" (i = 1 to 5) $F_{i,Ed,B}$ for one bolt or for one anchor is calculated as follows:

$F_{i,Ed,B} = k_{i,t,ax} \times F_{i,Ed}$ for axial stress on the bolt or anchor
$F_{i,Ed,B} = k_{i,t,v} \times F_{i,Ed}$ for lateral stress on the bolt or anchor

Load in one direction

In case of sole impact of load components F_1, $F_{2/3}$, $F4$ or F_5 or $F_{4/5}$ the proof of interaction must be provided in the following form:

3

$$\frac{F_{i,Ed}}{F_{i,Rd}} \leq 1$$

Load in several directions

In case of simultaneous impact of load components F_1, $F_{2/3}$, F_4 or F_5 and $F_{4/5}$ the proof of interaction must be provided as follows:
For a bracket, the loads F_4 and F_5 never take effect at the same time.

For one bracket:

$$\left(\frac{F_{1,Ed}}{F_{1,Rd}}\right)^2 + \left(\frac{F_{2/3,Ed}}{F_{2/3,Rd}}\right)^2 + \left(\frac{F_{4,Ed}}{F_{4,Rd}}\right)^2 + \left(\frac{F_{5,Ed}}{F_{5,Rd}}\right)^2 \leq 1$$

For two brackets:

$$\left(\frac{F_{1,Ed}}{F_{1,Rd}}\right)^2 + \left(\frac{F_{2/3,Ed}}{F_{2/3,Rd}}\right)^2 + \left(\frac{F_{4/5,Ed}}{F_{4/5,Rd}}\right)^2 \leq 1$$

General information

The load capacities apply to timbers with a characteristic raw density of 350 kg/m3.
Curvatures of the timber components and joist edges in the area of the angle brackets are not permitted - the wood must be sharp-edged in the area of the bracket.
There must be proof that no gaps occur in the timber component for all load directions according to EN 1995 or an equivalent national standard.

Minimum spacing according to EN 1995-1-1

[mm]		Force parallel to the grain	Force at a right angle to the grain	Force under the bracket a to the grain
		(α = 0°)	(α = 90°)	(α any)
a_1	in grain direction	28	14	(14+14 x cos α)
a_2	Right angle to the grain direction	14	14	14
$a_{3,t}$	End grain with stress	60	40	(40 + 20 x cos α)
$a_{3,c}$	End grain without stress	40	40	40
$a_{4,t}$	Loaded edge	20	28	(20 + 8 x sin α)
$a_{4,c}$	Unloaded edge	20	20	20

Nails Ø 4 mm, without pilot drilling, in nail plates, pk ≤ 420 kg/m³