

GH column base type T01 on concrete

ETA-16/0550



0769

General

Post supports are approved for service classes 1, 2 and 3.

Timber column

Softwood, C24 or higher strengths

Glulam

Minimum dimensions **min w x min h** see structural calculations table

Timber column fasteners

Wood screws

$\varnothing 8 \times 70 - l_{ef} \geq 50$ mm
 $\varnothing 10 \times 120 - l_{ef} \geq 100$ mm
 $\varnothing 10 \times 60, \varnothing 4 \times 60 - l_{ef} \geq 40$ mm
 $\varnothing 12 \times 80 - l_{ef} \geq 60$ mm

l_{ef} = minimum thread lengths

If screws with thread length l_{ef} greater than 100 mm are used, the resistance can be increased, see structural calculations table, index d)

Dowel

$\varnothing 8$ mm, $\varnothing 10$ mm and $\varnothing 12$ mm, at least S235

In concrete

The minimum concrete encased depth for concrete encased post supports is 150 mm.

Structural calculation tables

General

The table contains characteristic values of the resistance/load-carrying capacity for determining design values in ultimate limit state.

The resistances/load-carrying capacities apply to the maximum distances given in the structural calculation tables of the load application points from the top of the substrate.

The verification of anchoring of the post support in the subsoil must be provided separately.

In case of horizontal loading of the post support, it is recommended to verify the resistance with the lower value of the resistances F2/3 and F4/5, if correct layout of the post support in the place of installation is not checked.

Minimum and maximum distances

Distance from top of baseplate - top of substrate, see structural calculations table **max a**

$e_{2/3}$ - maximum distance between load application - top of substrate in load case F_{2/3}

$e_{4/5}$ - maximum distance between load application - top of substrate in load case F_{4/5}

The distances $e_{2/3}$ and $e_{4/5}$ result from the distance max a and the centre of gravity of the load application for the load cases F2/3 and F4/5.

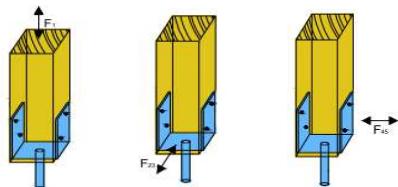
$$\sum F_{(i,Ed)} / F_{(i,Rd)} \leq 1$$

F_{1,c} u. F_{1,t}

F_{2/3}

F_{4/5}

F_{1,c} - compressive force (downwards) perpendicular to the baseplate
F_{1,t} - tensile force (upwards) perpendicular to the baseplate
F_{2/3} - load perpendicular to fasteners in the fin, dowel, ties
F_{4/5} - load parallel to fasteners in the fin, dowel, ties





„Innovationen im Holzbau“

Resistance design value

$$F_{i,Rd} = \min \{ k_{mod} \cdot F_{i,Rk,timber} / \gamma_{M,timber}; F_{i,Rk,Stahl} / \gamma_{M,steel} \}$$

with k_{mod} to EN 1995-1-1 and $\gamma_{M,timber} = 1.3$

For several connectors, 2 characteristic values are given for the steel load-carrying capacity with different partial safety factors $\gamma_M, steel$.

Both values are to be taken into consideration when determining the design value.

Resistance analysis

$$\sum \frac{F_{i,Ed}}{F_{i,Rd}} \leq 1$$

Indices

a) Resistance values apply to baseplates 8 mm and 6 mm thick.

b) Resistance values apply to a baseplate 8 mm thick. For a baseplate 6 mm thick, the values marked 1) to 6) are to be multiplied by the factor from the following table.

1)	2)	3)	4)	5)	6)
0,67	0,72	0,75	0,81	0,84	0,86

c) For tensile loading by load $F_{1,t}$, dowels are required in addition to the given screws.

d) If screws with threaded length l_{ef} greater than 100 mm are used, the load-carrying capacity $F_{1,t,Rk,timber}$ can be increased by factor $f_{1,t,timber} = (lef / 100 \text{ mm})0.9$.

Art.No.	Post		Maximum spacings			$F_{1,c}$ - compression			$F_{1,t}$ - tension			$F_{2/3}$			$F_{4/5}$		
	min w mm	min h mm	max a mm	$e_{2/3}$ mm	$e_{4/5}$ mm	Timber	Steel	γ_M	Timber	Steel	γ_M	Timber	Steel	γ_M	Timber	Steel	
						$F_{1,c,Rk}$	$F_{1,c,Rk}$	γ_M	$F_{1,t,Rk}$	$F_{1,t,Rk}$	γ_M	$F_{2/3,Rk}$	$F_{2/3,Rk}$	γ_M	$F_{4/5,Rk}$	$F_{4/5,Rk}$	γ_M
19812301 ^{a)}	100	100	70	180	92	75,6	108	1,10	24,8	6,88	1,00	9,22	1,87	1,00	2,24	4,61	1,00
19812302 ^{a)}			120	230	139	75,6	108	1,10	24,8	6,88	1,00	9,22	1,45	1,00	2,19	3,05	1,00
19812303 ^{a)}			200	310	214	75,6	108	1,10	24,8	6,88	1,00	9,22	1,07	1,00	2,10	1,98	1,00
19812638 ^{a)}			250	360	263	75,6	108	1,10	24,8	6,88	1,00	9,22	0,92	1,00	2,08	1,61	1,00
19812639 ^{a)}			300	410	312	75,6	108	1,10	24,8	6,88	1,00	9,22	0,81	1,00	1,99	1,36	1,00

4 dowels Ø10